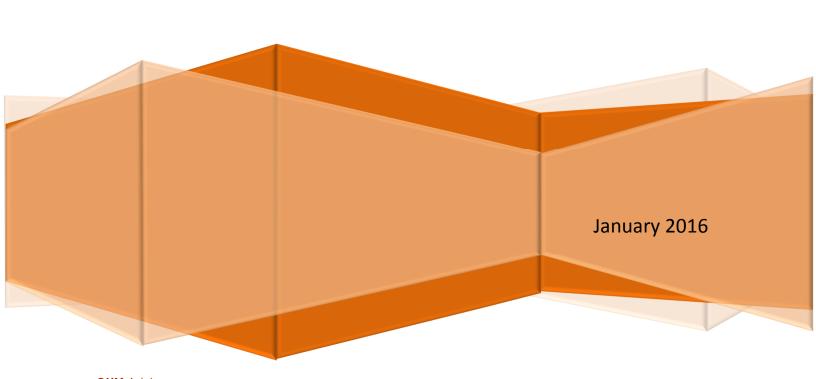


Stormwater Management Plan

Deer Run Subarea A and B

City of Dublin, Ohio





Stormwater Management Plan

Deer Run Subarea A and B

City of Dublin, Ohio

Prepared By:

OHM Advisors 580 North Fourth Street, Suite 610 Columbus, Ohio 43215

I hereby certify that the calculation contained herein are accurate to the best of my knowledge and belief.

Angola C. Fodak, B.F.

01.04.2016

Date



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I. Project Summary

Project Name: Deer Run Subarea A and B

Location: <u>Dublin, Ohio</u>

Type: <u>Stormwater Management Plan</u> Reviewing Agency: <u>City of Dublin</u>

Hydrological Analysis Method:

This report uses the unit hydrograph method described in the National Engineering Handbook (NEH) using the Natural Resources Conservation Service (NRCS) Type II 24-hr design storm. Requirements for the post construction quantity/quality control are per the City of Dublin Stormwater Management Design Manual, June 2013. Post construction quality control shall be designed according to the Ohio EPA General Permit Authorization for Storm Water Discharges Associated with Construction Activity Under the National Pollutant Discharge Elimination System (NPDES). HydroCAD Version 10.00 was the design software used for the analysis.

Rainfall data used (per the City of Dublin Stormwater Management Design Manual):

	Rainfall Depths									
1-yr	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
2.20 in.	2.63 in.	3.24 in.	3.74 in.	4.44 in.	5.02 in.	5.63 in.				

Proposed Design Overview:

Water Quantity: Not Applicable per the City of Dublin Stormwater Management Design Manual,

Chapter 2, Section 2.A.1.a

Water Quality: Bioretention Swale

Receiving Body of Water: Subarea A – Scioto River

Subarea B – Deer Run



II. Introduction

The design and analysis described within the contents of this Stormwater Management Report is for the construction project of Deer Run Road Subareas A and B in which private roads are being constructed to provide access to proposed estate lots located in this area. The overall project construction consists of two subareas, Subarea A and Subarea B. The overall development area for Subarea A is 18.5 acres and the overall development areas for Subarea B is 16.1 acres that will allow for the development of 7 total estate lots. The Deer Run Subarea A and B project development consists of the construction of the roadway to provide access to these proposed estate lots, still yet to be designed and developed. In Subarea A, the roadway construction consists of 0.69 acres of the overall 18.5 acres (1.87 acres are still available for estate development). In Subarea B, the roadway construction consists of 1.19 acres of the overall 16.1 acres (4.07 acres is still available for proposed estate lots development). The project is located off of Dublin Rd on the existing Deer Run Rd. with Subarea A located north of Deer Run and Subarea B located south of Deer Run. The intent of the project is to develop the existing wooded areas into several proposed estate lots for residential use.

The runoff from the roadway will be collected in a Bioretention swale to address quality requirements. In Subarea A, the water is collected in the bioretention swale and outlets to the Scioto River. In Subarea B, the runoff is collected in a bioretention swale and outlets to Deer Run.

III. Hydrologic Analysis Method

The design and analysis of the stormwater plan were completed using the unit hydrograph method described in the National Engineering Handbook (NEH) using the Natural Resources Conservation Service (NRCS) Type II 24-hr design storm. From the NRCS tables provided, the runoff curve number was determined as well as the method of calculating the time of concentration. From this, the 1, 2, 5, 10, 25, 50, and 100-year storm discharges were calculated using the HydroCAD (Version 10.00) program.

IV. Pre Developed Conditions

The overall pre-developed conditions for Subarea A consists of 18.5 acres to be rezoned. This land consists of wooded area with little to no ground cover. According to the United States Department of Agriculture (USDA) Soil Maps, this soil is Type "B" Soils (Milton silt loam, glynwood silt loam, and ground moraine) which corresponds to the runoff Curve Number 77. The time of concentration for the pre-developed area is 12.03 minutes.



The overall pre-developed condition for Subarea B consists of 16.1 acres to be rezoned. This land consists of wooded area with moderate ground cover in Type "D" Soils (Glynwood Clay Loam and End Moraine) which corresponds to the runoff Curve Number 83. The time of concentration for the pre-developed area is 5.08 minutes. Calculations for the time of concentration can be found in Appendix D. The information for the pre developed site conditions are shown in Table 1.

Table 1: Pre Developed Subarea Characteristics

	Area	Land Usage	Impervious	Weighted	Time of
	(acres)		Area (%)	CN	Concentration
					(minutes)
Subarea A	0.69	Wooded,	1.3%	77	12.03
		Impervious			
Subarea B	1.19	Wooded,	0.02%	83	5.08
		Impervious			

In subarea A, the pre developed area for the roadway is located in Sub-basin 10 of the Deer Run Watershed per the City of Dublin Stormwater Master Plan. Part of the roadway and proposed lot developments are located outside of this watershed in the unstudied areas and shall comply with the release rates specified in Chapter 2, Section C.1.b. This shall be considered when the separate construction plans and stormwater management plans are submitted for these development lots.

In Subarea B, the pre developed area for the roadway is located in Sub-basins 10 and 20 of the Deer Run Watershed per the City of Dublin Stormwater Master Plan. It should be noted that portions of 3 of the proposed estate lots are located outside of this watershed in the unstudied areas and shall comply with the release rates specified in Chapter 2, Section C.1.b. This shall be considered when the separate construction plans and stormwater management plans are submitted for these development lots. The approximate acreage of the proposed roadway development between the two sub-basins are listed in Table 2, as well as the respective allowable release rates.



Table 2: Allowable Release Rates

	Deer Run Watershed Release Rate Requirements (from the City of Dublin Stormwater Master Plan)											
	Design Storm (CFS/ac.)											
Sub-Basin	1-year	2-year	5-year	10-year	25-year	50-year	100-year					
10	0.10	0.10	0.10	0.30	0.80	1.80	2.80					
20	20 0.10 0.10 0.10 0.40 1.60 3.10 4.40											
Unstudied		< 2.0 acres, 0.20										

	Allowable Release Rates per Acre												
			Design Storm (CFS/ac.)										
Sub-Basin	Area (ac.)	1-year	2-year	5-year	10-year	25-year	50-year	100-year					
Subarea A													
10	0.10	0.01	0.01	0.01	0.03	0.08	0.18	0.28					
Unstudied	0.59	0.12	0.12	0.12	0.12	0.12	0.12	0.12					
Subarea B													
10	0.66	0.07	0.07	0.07	0.20	0.53	1.19	1.85					
20	0.47	0.05	0.05	0.05	0.19	0.75	1.46	2.07					

V. Post Developed Conditions

The proposed roadway construction area is shown in Exhibits 1 and 2. For Subarea A, this includes 0.69-acre development, of which 0.33 acres is comprised of the roadway with a gravel top course and the remaining development includes the proposed grading with the bioretention swales on both sides of the roadway. For subarea B, this includes a total of 1.19-acre development of which 0.47 acres is comprised of the roadway with a gravel top course and the remaining development is the proposed grading including the bioretention swales on both sides of the roadway. Composite runoff numbers were calculated in HydroCAD and are summarized in Table 3.

Exhibits 1 and 2 shows the design of the proposed roadway in Subarea A and B that will provide access to the future estate lots. The remaining estate lots will be designed and will provide separate construction design plans and stormwater management plans. This project will utilize a Bioretention Swale that runs parallel to the roadway and will provide quality treatment for the project.



Table 3: Post Developed Subarea Characteristics

	Area	Land Usage	Impervious	Weighted	Time of Concentration
	(acres)		Area (%)	CN	(minutes)
Subarea A	0.69	Impervious	48%	70	*Refer to the Time of
		Area, Wooded			Concentration Calcs in
		Area, Grass			Appendix D for the
		Cover			respective drainage
					areas.
Subarea B	1.19	Impervious	39%	84	*Refer to the Time of
		Area, Wooded			Concentration Calcs in
		Area, Grass			Appendix D for the
		Cover			respective drainage
					areas.

VI. Outlet Design

For Subarea A, there is only one outlet for the system. The bioretention swales are graded to the east and drains to the Scioto River.

For Subarea B, there are two outlets for the system. Due to the highpoint location, the proposed ditch could not be graded to the outfall located at the end of the cul-de-sac. Therefore, a proposed storm system was designed. The location of the proposed structures is at Station 1+00 and crosses perpendicular to the roadway. Two catch basins collect the ditch drainage into 12" conduits and outlet them to a roadway ditch that drains to Deer Run. The other outlet for the system is east of the cul-de-sac where the bioretention swale is graded to Deer Run.

VII. Maintenance and Inspection

The city shall be responsible for the inspection and maintenance of the bioretention swales located alongside the proposed roadway. Inspections and maintenance that are conducted shall be documented by the city. For initial sediment control, please refer to the City of Dublin, Stormwater Management Design Manual, Chapter 8 and the Sediment and Erosion control submitted with the proposed construction plans. The following procedures should be followed for the bioretention swales. Inspection of the storm structures to remove debris or sediment that has accumulated at the catch basins or outlet shall occur once a year. Maintenance (mowing or removal of excess sediment or debris) of the bioretention swales shall occur at a



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minimum on a monthly basis or more frequently if needed. More frequent maintenance may need required during the months of April through September, or during heavy rainfalls where runoff might carry debris into the swale and where grass might grow at a much higher rate. Adjustments may be made to this plan upon review of the 1^{st} year inspection results.

VIII. Post Construction Water Quality

The project is a new construction project so water quality treatment is required for a 0.75-inch event. Water Quantity treatment is not required since the project location is located within the River Corridor as per the City of Dublin Stormwater Management Design Manual, Chapter 2, Section 2.A.1.a. The % of impervious area for Subarea A is 1.3% pre development and is 48% post development. For Subarea B it was 0.02% pre development and is 39% post development. By utilizing Vegetated Bioretention Swale alongside the roadway this will allow the proposed soil media to improve the water quality and will also reduce runoff rates, even though quantity treatment is not required. The 12"soil layer under the finished grade of the ditch bottom, provides enhanced infiltration and pollutant removal. Therefore, we are able to treat the required 0.75-inch event for the project. Calculations that show the required volume storage to treat this rainfall event and the storage provided for each bioretention swale are shown in the calculations for the water quality conditions can be found in Appendix B.

Appendix A Storm Sewer Calculation Sheets

STRUC	TURE DA	TA				PIPE CA	LCULATI	ONS											ORM (year) = FFICIENT n =		HYDRAU	JLIC GRADE	E CALCULATI	ONS			PERCENT FULI	GRADE (year) = L AT OUTLET = R ELEVATION =	93%
STRUC. NO.	STRUC. TYPE	TOP OF CASTING FT.	INVERT IN ELEV. FT.	INVERT OUT ELEV. FT.	DEPTH OF STRUC. FT.	A ACRES	SUM A ACRES	AINAGE A C	REA CA ACRES	SUM CA ACRES	TI T MIN.	ME SUM T MIN.	RAINFALL INTENSITY I IN. / HR.	Q C.F.S.	OF PIPE	SLOPE OF PIPE FT. / FT.	SIZE OF PIPE IN.	MEAN VELOCITY F.P.S.	CAPACITY FLOWING FULL C.F.S.		TIME SUM T MIN.	HYDRAULIC RAINFALL INTENSITY I IN. / HR.	HYDRAULIC DISCHARGE Q C.F.S.	HYDRAULIC GRADE SLOPE FT. / FT.	HEAD LOSS COEFF. K	MINOR HEAD LOSS Hm FT.	HYDRAULIC GRADE DOWNHILL ELEV. FT.	HYDRAULIC GRADE UPHILL ELEV. FT.	DEPTH OF HYDRAULIC GRADE FT.
CB RT		850.42		848.30	2.12	0.680	0.680	0.55	0.374	0.374	10															0.0000	849.44		0.98
												10.00	6.98	2.61	38.00	0.0054	12	3.62	2.84	91.79	10.00	6.98	2.61	0.0045					
CB LT		850.35	848.09	848.09	2.26	0.150	0.830	0.55	0.083	0.457	0.17															0.0000	0.71	849.71	0.64
												10.17	6.98	3.19	41.00	0.0079	12	4.38	3.44	92.63	10.17	0.00	0.00	0.0000					
													541.05	2.22			4-				2.22	541.05	2.22	0.0000				0.71	
													FALSE	0.00			15				0.00	FALSE	0.00	0.0000				\longrightarrow	
	1	T	I	l	T T	Т			T T	Ι	I		1					Т	T	Т		Π				I	1		

Appendix B

Post- Construction Water Quality Calculations

Functional Classification: Local Road

Subarea A LT DITCH

Stormwater Management Requirements
Location of Project: River Corridor
Quantity Requirements: Not applicable
Quality Requirements: 0.75 inch event

Type of Drainage System: Open Channel

Hydrologic Soil Group: B

Ditches shall be designed to carry the 10 year, 24 hr storm.

Open Channels shall be designed with one foot of freeboard above the design water surface elevation for the 10 year, 24-hour storm.

The 100 yr, 24 hour storm shall not encroach into proposed or existing residential dwellings or places of business.

Method of treating water

quality for the project:

Bioretention Swale

Pre Construction 0.00 acres

Imprevious Area

Post Construction
Impervious Area

Total Area

0.10 acres

0.27 acres

37.06%

Time of Concentration

tc = 0.13 hr

Water Quality Volume (WQv)

 $WQ_V = C * (P/12) * A$ Where, $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

 $i = fraction \ of \ post-const. impervious \ surface$ $C = 0.26339689 \qquad P = 0.75 Precipitation \ Depth$

 $WQ_V = 0.0045$ ac-ft 194.2619 CU FT

acre

 WQ_V Elevation = 0.19 ft water depth in swale

0.27

Available Depth in Ditch: 1.55 FT Available Storage in Ditch: 2,062 CF

Functional Classification: Local Road

Subarea A RT DITCH

Stormwater Management Requirements

Location of Project:River CorridorQuantity Requirements:Not applicableQuality Requirements:0.75 inch event

Type of Drainage System: Open Channel

Hydrologic Soil Group: B

Ditches shall be designed to carry the 10 year, 24 hr storm.

Open Channels shall be designed with one foot of freeboard above the design water surface elevation for the 10 year, 24-hour storm.

The 100 yr, 24 hour storm shall not encroach into proposed or existing residential dwellings or places of business.

Method of treating water

quality for the project:

Bioretention Swale

Pre Construction

Imprevious Area 0.009 acres

Post Construction

Impervious Area 0.229 acres

Total Area 0.7253 acres % of impervious area 31.57%

Time of Concentration

tc = 0.24 hr

Water Quality Volume (WQv)

 $WQ_V = C * {P/12} * A$ Where, C = 0.8

 $Q_V = C * (^1/12) * A$ $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

i = fraction of post - const. impervious surface

C = 0.23363 P = 0.75Precipitation Depth A = Area tributary to basin, acres

P = 0.75 inches A = 0.7253 acre

 $WQ_V = 0.01059$ ac-ft 461.3239 CU FT

 WQ_V Elevation = 0.16 ft water depth in swale

Available Depth in Ditch: 1.55 FT Available Storage in Ditch: 3,010 CF

Functional Classification: Local Road

Subarea B RT DITCH #1

Stormwater Management Requirements

Location of Project: River Corridor

Quantity Requirements: Not applicable

Quality Requirements: 0.75 inch event

Type of Drainage System: Open Channel

Hydrologic Soil Group: D

Ditches shall be designed to carry the 10 year, 24 hr storm.

Open Channels shall be designed with one foot of freeboard above the design water surface elevation for the 10 year, 24-hour storm.

The 100 yr, 24 hour storm shall not encroach into proposed or existing residential dwellings or places of business.

Method of treating water

quality for the project:

Bioretention Swale

Pre Construction

Imprevious Area 0.022 acres

Post Construction

Impervious Area

Total Area

0.083 acres
0.683 acres

% of impervious area 12.15%

Time of Concentration

tc = 0.12 hr

Water Quality Volume (WQv)

 $WQ_V = C * (P/_{12}) * A$ Where, C = 0.8

 $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

 $i = fraction \ of \ post-const.$ impervious surface

C = 0.12408 P = 0.75 Precipitation Depth A = Area tributary to basin, acres

P = 0.75 inches A = 0.6811 acre

 $WQ_V = 0.00528$ ac-ft 230.0799 CU FT

 WQ_V Elevation = 0.16 ft water depth in swale

Available Depth in Ditch: 1.55 FT

Available Storage in Ditch: 2,765 CU FT

Functional Classification: Local Road

Subarea B RT DITCH #2

Stormwater Management Requirements
Location of Project: River Corridor
Quantity Requirements: Not applicable
Quality Requirements: 0.75 inch event

Type of Drainage System: Open Channel

Hydrologic Soil Group: D

Ditches shall be designed to carry the 10 year, 24 hr storm.

Open Channels shall be designed with one foot of freeboard above the design water surface elevation for the 10 year, 24-hour storm.

The 100 yr, 24 hour storm shall not encroach into proposed or existing residential dwellings or places of business.

Method of treating water

quality for the project:

Bioretention Swale

Pre Construction Imprevious

0.00 acres

Post Construction

Impervious Area 0.18 acres

Total Area 0.80 acres % of impervious area 22.03%

Time of Concentration

tc = 0.24 hr

Water Quality Volume (WQv)

 $WQ_V = C * (P/_{12}) * A$ Where, $C = 0.858i^3 - 0.78i^2$

* $(^{1}/_{12})$ * A $C = 0.858i^{3} - 0.78i^{2} + 0.774i + 0.04$

 $i = fraction \ of \ post - const. impervious \ surface$

C = 0.181822 P = 0.75Precipitation Depth A = Area tributary to basin, acres

P = 0.75 inches A = 0.8027 acre

 $WQ_V = 0.009122$ ac-ft 397.34 CU FT

 WQ_V Elevation = 0.10 ft water depth in swale

Available Depth in Ditch: 1.55 FT Available Storage in Ditch: 50,490 CF

Functional Classification: Local Road

Subarea B LT DITCH #1

Stormwater Management Requirements
Location of Project: River Corridor
Quantity Requirements: Not applicable
Quality Requirements: 0.75 inch event

Type of Drainage System: Open Channel

Hydrologic Soil Group: D

Ditches shall be designed to carry the 10 year, 24 hr storm.

Open Channels shall be designed with one foot of freeboard above the design water surface elevation for the 10 year, 24-hour storm.

The 100 yr, 24 hour storm shall not encroach into proposed or existing residential dwellings or places of business.

Method of treating water quality for the project:

Bioretention Swale

Pre Construction Imprevious

0.006 acres

Post Construction

Impervious Area 0.063 acres

Total Area 0.1534 acres % of impervious area 41.07%

Time of Concentration

tc = 0.17 hr

Water Quality Volume (WQv)

 $WQ_V = C * (P/_{12}) * A$ Where, C = 0.85

 $C = 0.858i^{3} - 0.78i^{2} + 0.774i + 0.04$

 $i = fraction \ of \ post - const. impervious \ surface$

C = 0.285748 P = 0.75Precipitation Depth A = Area tributary to basin, acres

P = 0.75 inches A = 0.1534 acre

 $WQ_V = 0.00274$ ac-ft 119.3375 CU FT

 WQ_V Elevation = 0.10 ft water depth in swale

Available Depth in Ditch: 1.55 FT

Available Storage in Ditch: 5(3,162 CF

Functional Classification: Local Road

Subarea B LT DITCH #2

Stormwater Management Requirements

Location of Project: River Corridor Quantity Requirements: Not applicable **Quality Requirements:** 0.75 inch event

Type of Drainage System: **Open Channel**

Hydrologic Soil Group: D

Ditches shall be designed to carry the 10 year, 24 hr storm.

Open Channels shall be designed with one foot of freeboard above the design water surface elevation for the 10 year, 24-hour storm.

The 100 yr, 24 hour storm shall not encroach into proposed or existing residential dwellings or places of business.

Method of treating water

Bioretention Swale quality for the project:

Pre Construction

Imprevious Area

0 acres

Post Construction

0.1485 acres

Impervious Area

Total Area

0.3471 acres

% of impervious area

42.78%

Time of Concentration

tc = 0.33 hr

Water Quality Volume (WQv)

 $WQ_V = C * (P/_{12}) * A$

 $C = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$

 $i = fraction \ of \ post - const. impervious \ surface$

C = 0.2956I = 0.4278 P = 0.75Precipitation Depth A = Area tributary to basin, acres

P = 0.75 inches

A = 0.3471 acre

279.30 CU FT

 WQ_V Elevation = 0.08 ft water depth in swale

0.0064 ac-ft

Available Depth in Ditch: 1.55 FT

Available Storage in Ditch: 5 6,798 CF

 $WQ_V =$

Appendix C HydroCAD Reports

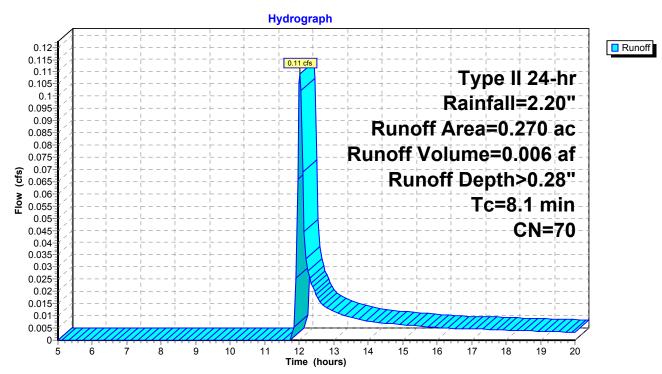
Summary for Subcatchment 3S: Post Const. Runoff Subarea A LT Ditch

Runoff = 0.11 cfs @ 12.02 hrs, Volume= 0.006 af, Depth> 0.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

Area	a (ac)	CN	Desc	cription		
	0.170	61	>75%	6 Grass co	over, Good	I, HSG B
(0.100	85	Grav	el roads, l	HSG B	
	0.270	70	Weig	hted Aver	age	
(0.270		100.	00% Pervi	ous Area	
To	Leng	jth :	Slope	Velocity	Capacity	Description
(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
8.1						Direct Entry,

Subcatchment 3S: Post Const. Runoff Subarea A LT Ditch



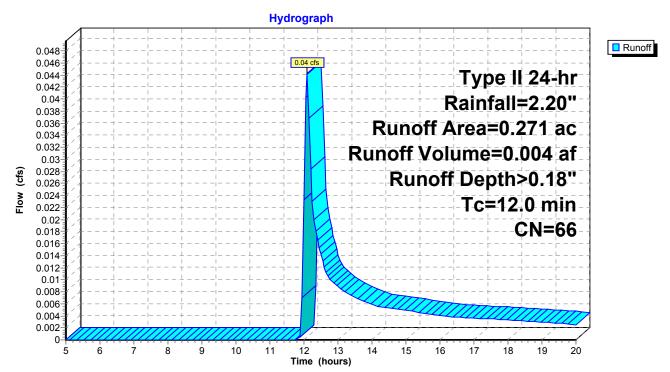
Summary for Subcatchment 1S: Pre Const. Runoff Subarea A LT Ditch

Runoff = 0.04 cfs @ 12.09 hrs, Volume= 0.004 af, Depth> 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

	Area	(ac)	CN	Desc	cription		
	0.	271	66	Woo	ds, Poor, I	HSG B	
-	0.	271		100.	00% Pervi	ous Area	
	Tc	Leng	jth :	Slope	Velocity	Capacity	Description
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	12 0						Direct Entry

Subcatchment 1S: Pre Const. Runoff Subarea A LT Ditch



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This report was prepared with the free HydroCAD SAMPLER, which is licensed for evaluation and educational use only. For actual design or modeling applications you must use a full version of HydroCAD which may be purchased at www.hydrocad.net. Full programs also include complete documentation, technical support, training materials, and additional features which are essential for actual design work.

Hydrograph for Subcatchment 1S: Pre Const. Runoff Subarea A LT Ditch

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00	0.18	0.00	0.00
6.25 6.50	0.19 0.20	0.00	0.00 0.00
6.75	0.20	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25 8.50	0.28 0.29	0.00	0.00 0.00
8.75	0.29	0.00	0.00
9.00	0.32	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25 10.50	0.42 0.45	0.00	0.00 0.00
10.50	0.43	0.00	0.00
11.00	0.52	0.00	0.00
11.25	0.56	0.00	0.00
11.50	0.62	0.00	0.00
11.75	0.85	0.00	0.00
12.00	1.46	0.03	0.02
12.25 12.50	1.55 1.62	0.05 0.06	0.02 0.01
12.75	1.66	0.00	0.01
13.00	1.70	0.08	0.01
13.25	1.73	0.08	0.01
13.50	1.76	0.09	0.01
13.75	1.78	0.10	0.01
14.00	1.80	0.10	0.01
14.25 14.50	1.82 1.84	0.11 0.11	0.01 0.01
14.75	1.86	0.11	0.01
15.00	1.88	0.12	0.00
15.25	1.89	0.12	0.00
15.50	1.91	0.13	0.00
15.75	1.92	0.13	0.00
16.00	1.94	0.14	0.00
16.25	1.95	0.14	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.14	0.00
16.75	1.97	0.15	0.00
17.00	1.98	0.15	0.00
17.25	1.99	0.15	0.00
17.50	2.01	0.16	0.00
17.75	2.02	0.16	0.00
18.00	2.03	0.16	0.00
18.25	2.04	0.16	0.00
18.50	2.05	0.17	0.00
18.75	2.05	0.17	0.00
19.00	2.06	0.17	0.00
19.25	2.07	0.18	0.00
19.50	2.08	0.18	0.00
19.75	2.09	0.18	0.00
20.00	2.09	0.18	0.00

Hydrograph for Subcatchment 3S: Post Const. Runoff Subarea A LT Ditch

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00 6.25	0.18 0.19	0.00	0.00 0.00
6.50	0.19	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50 8.75	0.29 0.31	0.00	0.00 0.00
9.00	0.31	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48 0.52	0.00	0.00
11.00 11.25	0.52	0.00	0.00 0.00
11.50	0.62	0.00	0.00
11.75	0.85	0.00	0.00
12.00	1.46	0.07	0.11
12.25	1.55	0.10	0.03
12.50	1.62	0.11	0.02
12.75	1.66	0.13	0.01
13.00	1.70	0.14	0.01
13.25 13.50	1.73 1.76	0.15 0.16	0.01 0.01
13.75	1.78	0.16	0.01
14.00	1.80	0.17	0.01
14.25	1.82	0.18	0.01
14.50	1.84	0.18	0.01
14.75	1.86	0.19	0.01
15.00	1.88	0.20	0.01
15.25	1.89	0.20	0.01
15.50 15.75	1.91	0.21 0.21	0.01
16.00	1.92 1.94	0.21	0.01 0.01
16.25	1.95	0.22	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.23	0.00
16.75	1.97	0.23	0.00
17.00	1.98	0.23	0.00
17.25	1.99	0.24	0.00
17.50	2.01	0.24	0.00
17.75	2.02	0.25	0.00
18.00	2.03	0.25	0.00
18.25	2.04	0.25	0.00
18.50	2.05	0.26	0.00
18.75	2.05	0.26	0.00
19.00	2.06	0.26	0.00
19.25	2.07	0.27	0.00
19.50	2.08	0.27	0.00
19.75	2.09	0.27	0.00
20.00	2.09	0.28	0.00

Deer Run A_LT

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Summary for Reach 4R: Biofilter

Inflow Area = 0.156 ac, 0.00% Impervious, Inflow Depth > 0.30" Inflow = 0.07 cfs @ 12.01 hrs, Volume= 0.004 af

Outflow = 0.07 cfs @ 12.10 hrs, Volume= 0.004 af, Atten= 10%, Lag= 5.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 1.22 fps, Min. Travel Time= 2.7 min

Avg. Velocity = 0.84 fps, Avg. Travel Time= 2.7 min

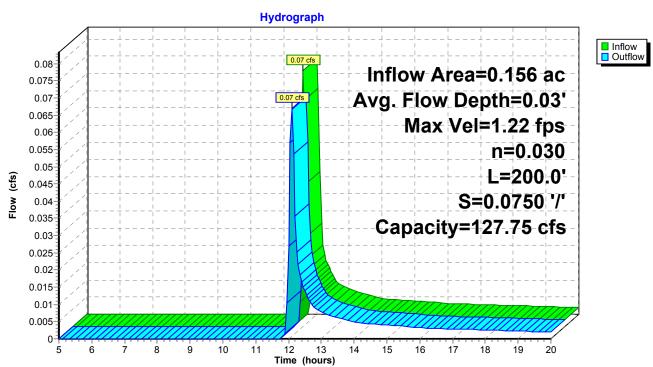
Peak Storage= 11 cf @ 12.05 hrs Average Depth at Peak Storage= 0.03'

Bank-Full Depth= 1.55' Flow Area= 10.3 sf, Capacity= 127.75 cfs

2.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 3.0 '/' Top Width= 11.30' Length= 200.0' Slope= 0.0750 '/' Inlet Invert= 830.00', Outlet Invert= 815.00'



Reach 4R: Biofilter



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Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	830.00	0.00
5.50	0.00	0	830.00	0.00
6.00	0.00	0	830.00	0.00
6.50	0.00	0	830.00	0.00
7.00	0.00	0	830.00	0.00
7.50	0.00	0	830.00	0.00
8.00	0.00	0	830.00	0.00
8.50	0.00	0	830.00	0.00
9.00	0.00	0	830.00	0.00
9.50	0.00	0	830.00	0.00
10.00	0.00	0	830.00	0.00
10.50	0.00	0	830.00	0.00
11.00	0.00	0	830.00	0.00
11.50	0.00	0	830.00	0.00
12.00	0.07	10	830.02	0.03
12.50	0.01	3	830.01	0.01
13.00	0.01	2	830.00	0.01
13.50	0.01	1	830.00	0.01
14.00	0.00	1	830.00	0.00
14.50	0.00	1	830.00	0.00
15.00	0.00	1	830.00	0.00
15.50	0.00	1	830.00	0.00
16.00	0.00	1	830.00	0.00
16.50	0.00	1	830.00	0.00
17.00	0.00	1	830.00	0.00
17.50	0.00	1	830.00	0.00
18.00	0.00	1	830.00	0.00
18.50	0.00	1	830.00	0.00
19.00	0.00	1	830.00	0.00
19.50	0.00	1	830.00	0.00
20.00	0.00	0	830.00	0.00

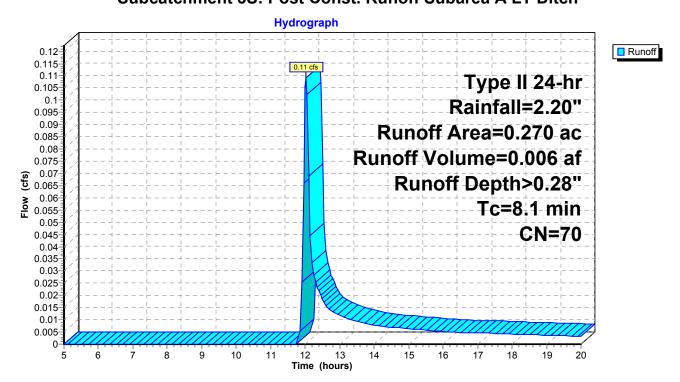
Summary for Subcatchment 3S: Post Const. Runoff Subarea A LT Ditch

Runoff = 0.11 cfs @ 12.02 hrs, Volume= 0.006 af, Depth> 0.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

Area	(ac)	CN	Desc	cription		
0	.170	61	>75%	6 Grass co	over, Good	I, HSG B
0	.100	85	Grav	el roads, l	HSG B	
0	0.270 70 Weighted Average				age	
0.	0.270 100.00% Pervious Area				ous Area	
Tc	Leng	jth :	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
8.1						Direct Entry,

Subcatchment 3S: Post Const. Runoff Subarea A LT Ditch



Hydrograph for Subcatchment 3S: Post Const. Runoff Subarea A LT Ditch

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00 6.25	0.18 0.19	0.00	0.00 0.00
6.50	0.19	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50 8.75	0.29 0.31	0.00	0.00 0.00
9.00	0.31	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48 0.52	0.00	0.00
11.00 11.25	0.52	0.00	0.00 0.00
11.50	0.62	0.00	0.00
11.75	0.85	0.00	0.00
12.00	1.46	0.07	0.11
12.25	1.55	0.10	0.03
12.50	1.62	0.11	0.02
12.75	1.66	0.13	0.01
13.00	1.70	0.14	0.01
13.25 13.50	1.73 1.76	0.15 0.16	0.01 0.01
13.75	1.78	0.16	0.01
14.00	1.80	0.17	0.01
14.25	1.82	0.18	0.01
14.50	1.84	0.18	0.01
14.75	1.86	0.19	0.01
15.00	1.88	0.20	0.01
15.25	1.89	0.20	0.01
15.50 15.75	1.91	0.21 0.21	0.01
16.00	1.92 1.94	0.21	0.01 0.01
16.25	1.95	0.22	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.23	0.00
16.75	1.97	0.23	0.00
17.00	1.98	0.23	0.00
17.25	1.99	0.24	0.00
17.50	2.01	0.24	0.00
17.75	2.02	0.25	0.00
18.00	2.03	0.25	0.00
18.25	2.04	0.25	0.00
18.50	2.05	0.26	0.00
18.75	2.05	0.26	0.00
19.00	2.06	0.26	0.00
19.25	2.07	0.27	0.00
19.50	2.08	0.27	0.00
19.75	2.09	0.27	0.00
20.00	2.09	0.28	0.00

Deer Run A_LT

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Summary for Reach 4R: Biofilter

Inflow Area = 0.270 ac, 0.00% Impervious, Inflow Depth > 0.28" Inflow = 0.11 cfs @ 12.02 hrs, Volume= 0.006 af

Outflow = 0.10 cfs @ 12.11 hrs, Volume= 0.006 af, Atten= 9%, Lag= 5.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.46 fps, Min. Travel Time= 2.9 min Avg. Velocity = 0.91 fps, Avg. Travel Time= 4.6 min

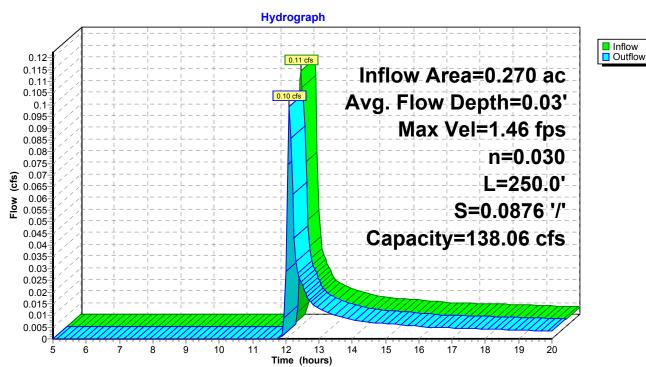
Peak Storage= 17 cf @ 12.06 hrs Average Depth at Peak Storage= 0.03'

Bank-Full Depth= 1.55' Flow Area= 10.3 sf, Capacity= 138.06 cfs

2.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 3.0 '/' Top Width= 11.30' Length= 250.0' Slope= 0.0876 '/' Inlet Invert= 831.60', Outlet Invert= 809.70'



Reach 4R: Biofilter



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Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	831.60	0.00
5.50	0.00	0	831.60	0.00
6.00	0.00	0	831.60	0.00
6.50	0.00	Ő	831.60	0.00
7.00	0.00	Ő	831.60	0.00
7.50	0.00	0	831.60	0.00
8.00	0.00	Ö	831.60	0.00
8.50	0.00	0	831.60	0.00
9.00	0.00	Ö	831.60	0.00
9.50	0.00	0	831.60	0.00
10.00	0.00	0	831.60	0.00
10.50	0.00	0	831.60	0.00
11.00	0.00	0	831.60	0.00
11.50	0.00	0	831.60	0.00
12.00	0.11	14	831.63	0.03
12.50	0.02	6	831.61	0.02
13.00	0.01	4	831.61	0.01
13.50	0.01	3	831.61	0.01
14.00	0.01	2	831.60	0.01
14.50	0.01	2 2	831.60	0.01
15.00	0.01	2	831.60	0.01
15.50	0.01	2	831.60	0.01
16.00	0.01	1	831.60	0.01
16.50	0.00	1	831.60	0.00
17.00	0.00	1	831.60	0.00
17.50	0.00	1	831.60	0.00
18.00	0.00	1	831.60	0.00
18.50	0.00	1	831.60	0.00
19.00	0.00	1	831.60	0.00
19.50	0.00	1	831.60	0.00
20.00	0.00	1	831.60	0.00

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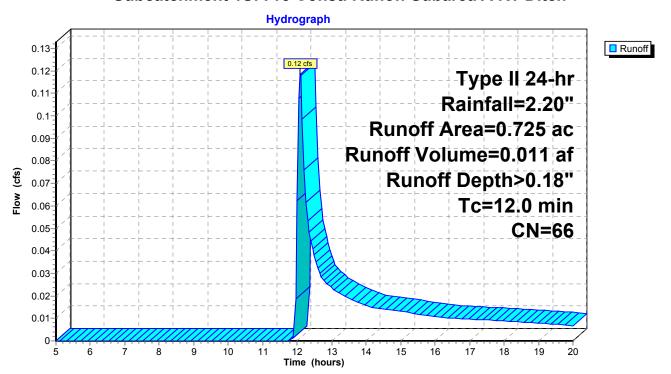
Summary for Subcatchment 1S: Pre Const. Runoff Subarea A RT Ditch

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 0.011 af, Depth> 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

_	Area	(ac)	CN	Desc	cription		
	0.	.725	66	Woo	ds, Poor, I	HSG B	
_	0.	.725		100.	00% Pervi	ous Area	
	_					_	
	Tc	Leng	gth	Slope	Velocity	Capacity	Description
_	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
-	12 0						Direct Entry

Subcatchment 1S: Pre Const. Runoff Subarea A RT Ditch



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Hydrograph for Subcatchment 1S: Pre Const. Runoff Subarea A RT Ditch

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00	0.18	0.00	0.00
6.25	0.19	0.00	0.00
6.50 6.75	0.20	0.00	0.00
7.00	0.21 0.22	0.00	0.00 0.00
7.00	0.22	0.00	0.00
7.50	0.23	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50	0.29	0.00	0.00
8.75	0.31	0.00	0.00
9.00	0.32	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48	0.00	0.00
11.00	0.52	0.00	0.00
11.25	0.56	0.00	0.00
11.50 11.75	0.62	0.00	0.00
12.00	0.85 1.46	0.00	0.00 0.06
12.00	1.40	0.05	0.06
12.50	1.62	0.05	0.04
12.75	1.66	0.07	0.03
13.00	1.70	0.08	0.02
13.25	1.73	0.08	0.02
13.50	1.76	0.09	0.02
13.75	1.78	0.10	0.02
14.00	1.80	0.10	0.02
14.25	1.82	0.11	0.01
14.50	1.84	0.11	0.01
14.75	1.86	0.12	0.01
15.00	1.88	0.12	0.01
15.25	1.89	0.12	0.01
15.50	1.91	0.13	0.01
15.75	1.92	0.13 0.14	0.01
16.00 16.25	1.94	0.14	0.01
10.25	1.95	0.14	0.01

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.14	0.01
16.75	1.97	0.15	0.01
17.00	1.98	0.15	0.01
17.25	1.99	0.15	0.01
17.50	2.01	0.16	0.01
17.75	2.02	0.16	0.01
18.00	2.03	0.16	0.01
18.25	2.04	0.16	0.01
18.50	2.05	0.17	0.01
18.75	2.05	0.17	0.01
19.00	2.06	0.17	0.01
19.25	2.07	0.18	0.01
19.50	2.08	0.18	0.01
19.75	2.09	0.18	0.01
20.00	2.09	0.18	0.01

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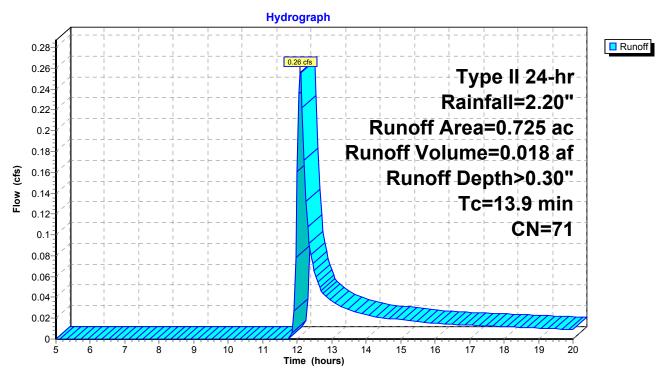
Summary for Subcatchment 3S: Post Const. Runoff Subarea A RT Ditch

Runoff = 0.26 cfs @ 12.09 hrs, Volume= 0.018 af, Depth> 0.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

Area	a (ac)	CN	Desc	ription		
(0.305	66	Woo	ds, Poor, I	HSG B	
(0.229	85	Grav	el roads, l	HSG B	
(0.191	61	>75%	√ Grass co	over, Good	I, HSG B
(0.725 71 Weighted Average					
(0.725 100.00% Pervious Area			00% Pervi	ous Area	
_						
To		•	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
13.9						Direct Entry, Tc

Subcatchment 3S: Post Const. Runoff Subarea A RT Ditch



Hydrograph for Subcatchment 3S: Post Const. Runoff Subarea A RT Ditch

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00	0.18	0.00	0.00
6.25	0.19	0.00	0.00
6.50	0.20	0.00	0.00
6.75	0.21	0.00	0.00
7.00 7.25	0.22 0.23	0.00	0.00
7.50	0.23	0.00	0.00 0.00
7.30	0.24	0.00	0.00
8.00	0.25	0.00	0.00
8.25	0.28	0.00	0.00
8.50	0.29	0.00	0.00
8.75	0.31	0.00	0.00
9.00	0.32	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48	0.00	0.00
11.00	0.52	0.00	0.00
11.25	0.56	0.00	0.00
11.50	0.62	0.00	0.00
11.75	0.85	0.00	0.00
12.00	1.46	0.09	0.16
12.25	1.55	0.11	0.13
12.50	1.62	0.13	0.07
12.75	1.66	0.14	0.04
13.00 13.25	1.70 1.73	0.16 0.17	0.04 0.03
13.50	1.73	0.17	0.03
13.75	1.78	0.18	0.03
14.00	1.80	0.10	0.03
14.25	1.82	0.10	0.02
14.50	1.84	0.21	0.02
14.75	1.86	0.21	0.02
15.00	1.88	0.22	0.02
15.25	1.89	0.22	0.02
15.50	1.91	0.23	0.02
15.75	1.92	0.24	0.02
16.00	1.94	0.24	0.02
16.25	1.95	0.25	0.01

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.25	0.01
16.75	1.97	0.25	0.01
17.00	1.98	0.26	0.01
17.25	1.99	0.26	0.01
17.50	2.01	0.27	0.01
17.75	2.02	0.27	0.01
18.00	2.03	0.28	0.01
18.25	2.04	0.28	0.01
18.50	2.05	0.28	0.01
18.75	2.05	0.29	0.01
19.00	2.06	0.29	0.01
19.25	2.07	0.29	0.01
19.50	2.08	0.30	0.01
19.75	2.09	0.30	0.01
20.00	2.09	0.30	0.01

Deer Run A _RT

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Summary for Reach 4R: Biofilter

Inflow Area = 0.725 ac, 0.00% Impervious, Inflow Depth > 0.30" Inflow = 0.26 cfs @ 12.09 hrs, Volume= 0.018 af

Outflow = 0.24 cfs @ 12.17 hrs, Volume= 0.018 af, Atten= 4%, Lag= 4.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.03 fps, Min. Travel Time= 2.5 min Avg. Velocity = 0.96 fps, Avg. Travel Time= 5.2 min

Peak Storage= 36 cf @ 12.12 hrs Average Depth at Peak Storage= 0.06' Bank-Full Depth= 1.55' Flow Area= 10.3 sf, Capacity= 138.06 cfs

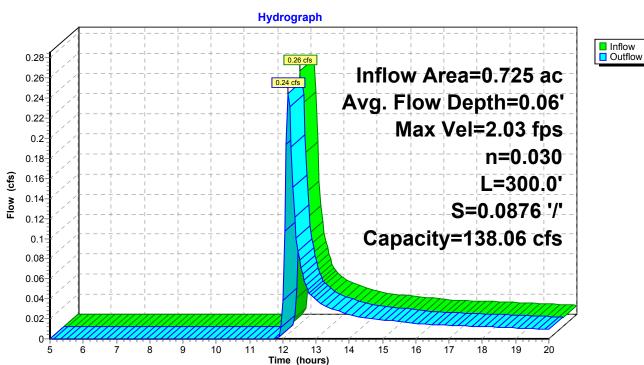
2.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 3.0 '/' Top Width= 11.30'

Length= 300.0' Slope= 0.0876 '/'

Inlet Invert= 831.60', Outlet Invert= 805.32'



Reach 4R: Biofilter



Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	831.60	0.00
5.50	0.00	0	831.60	0.00
6.00	0.00	0	831.60	0.00
6.50	0.00	0	831.60	0.00
7.00	0.00	0	831.60	0.00
7.50	0.00	0	831.60	0.00
8.00	0.00	0	831.60	0.00
8.50	0.00	0	831.60	0.00
9.00	0.00	0	831.60	0.00
9.50	0.00	0	831.60	0.00
10.00	0.00	0	831.60	0.00
10.50	0.00	0	831.60	0.00
11.00	0.00	0	831.60	0.00
11.50	0.00	0	831.60	0.00
12.00	0.16	20	831.63	0.04
12.50	0.07	17	831.63	0.08
13.00	0.04	11	831.62	0.04
13.50	0.03	10	831.62	0.03
14.00	0.02	8	831.61	0.02
14.50	0.02	7	831.61	0.02
15.00	0.02	6	831.61	0.02
15.50	0.02	6	831.61	0.02
16.00	0.02	5	831.61	0.02
16.50	0.01	5	831.61	0.01
17.00	0.01	4	831.61	0.01
17.50	0.01	4	831.61	0.01
18.00	0.01	4	831.61	0.01
18.50	0.01	4	831.61	0.01
19.00	0.01	4	831.61	0.01
19.50	0.01	3	831.61	0.01
20.00	0.01	3	831.61	0.01

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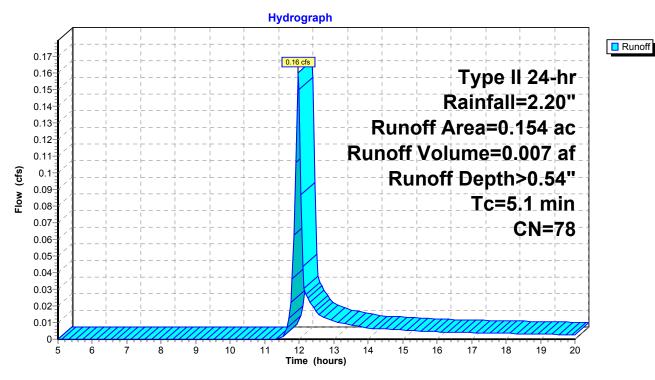
Summary for Subcatchment 1S: Pre Const. Runoff Subarea B LT Ditch #1

Runoff = 0.16 cfs @ 11.97 hrs, Volume= 0.007 af, Depth> 0.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

	Area	(ac)	CN	Desc	cription		
	0.	0.148 77 Woods, Good, HSG D				HSG D	
	0.006 91 Gravel roads, HSG D 0.154 78 Weighted Average				vel roads, l	HSG D	
					ghted Aver	age	
	0.154 100.00% Pervious Area					ous Area	
	Тс	Leng	jth	Slope	Velocity	Capacity	Description
((min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	5.1						Direct Entry,

Subcatchment 1S: Pre Const. Runoff Subarea B LT Ditch #1



Hydrograph for Subcatchment 1S: Pre Const. Runoff Subarea B LT Ditch #1

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00 6.25	0.18 0.19	0.00	0.00 0.00
6.50	0.19	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50 8.75	0.29 0.31	0.00	0.00 0.00
9.00	0.31	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48 0.52	0.00	0.00
11.00 11.25	0.52	0.00	0.00 0.00
11.50	0.62	0.00	0.00
11.75	0.85	0.03	0.02
12.00	1.46	0.22	0.15
12.25	1.55	0.26	0.02
12.50	1.62	0.29	0.02
12.75	1.66	0.31	0.01
13.00	1.70	0.33	0.01
13.25 13.50	1.73 1.76	0.34 0.35	0.01 0.01
13.75	1.78	0.33	0.01
14.00	1.80	0.38	0.01
14.25	1.82	0.39	0.01
14.50	1.84	0.40	0.01
14.75	1.86	0.41	0.01
15.00	1.88	0.42	0.01
15.25	1.89	0.43	0.01
15.50	1.91	0.43	0.00
15.75 16.00	1.92 1.94	0.44 0.45	0.00 0.00
16.25	1.95	0.46	0.00

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.46	0.00
16.75	1.97	0.47	0.00
17.00	1.98	0.48	0.00
17.25	1.99	0.48	0.00
17.50	2.01	0.49	0.00
17.75	2.02	0.49	0.00
18.00	2.03	0.50	0.00
18.25	2.04	0.50	0.00
18.50	2.05	0.51	0.00
18.75	2.05	0.52	0.00
19.00	2.06	0.52	0.00
19.25	2.07	0.52	0.00
19.50	2.08	0.53	0.00
19.75	2.09	0.53	0.00
20.00	2.09	0.54	0.00

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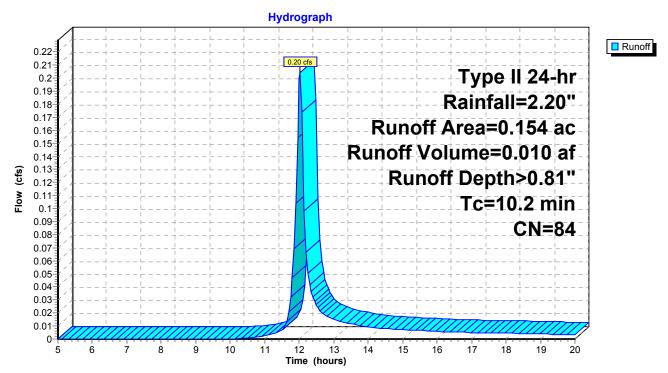
Summary for Subcatchment 3S: Post Const. RunoffSubarea B LT Ditch #1

Runoff = 0.20 cfs @ 12.02 hrs, Volume= 0.010 af, Depth> 0.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

	Area	(ac)	CN	Desc	cription			
	0.	009	77	Woo	ds, Good,	HSG D		
	0.	063	91	Grav	el roads, l	HSG D		
	0.	082	80	>759	% Grass co	over, Good	, HSG D	
	0.154 84 Weighted Average				ghted Aver	age		
	0.	0.154 100.00% Pervious Area				ous Area		
	Tc	Leng		Slope	Velocity	Capacity	Description	
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
	10.2						Direct Entry To	

Subcatchment 3S: Post Const. RunoffSubarea B LT Ditch #1



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Hydrograph for Subcatchment 3S: Post Const. RunoffSubarea B LT Ditch #1

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00	0.18	0.00	0.00
6.25 6.50	0.19 0.20	0.00	0.00
6.75	0.20	0.00	0.00 0.00
7.00	0.21	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50	0.29	0.00	0.00
8.75	0.31	0.00	0.00
9.00	0.32	0.00	0.00
9.25 9.50	0.34 0.36	0.00	0.00 0.00
9.50	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48	0.00	0.00
11.00	0.52	0.01	0.00
11.25	0.56	0.02	0.00
11.50	0.62	0.03	0.01
11.75	0.85	0.09	0.03
12.00 12.25	1.46 1.55	0.39 0.45	0.20 0.05
12.23	1.62	0.49	0.03
12.75	1.66	0.51	0.02
13.00	1.70	0.54	0.02
13.25	1.73	0.56	0.01
13.50	1.76	0.58	0.01
13.75	1.78	0.59	0.01
14.00	1.80	0.61	0.01
14.25	1.82	0.62	0.01
14.50	1.84	0.63	0.01
14.75 15.00	1.86 1.88	0.65 0.66	0.01 0.01
15.00	1.89	0.67	0.01
15.50	1.91	0.67	0.01
15.75	1.92	0.69	0.01
16.00	1.94	0.70	0.01
16.25	1.95	0.71	0.01

Precip.	Excess	Runoff
(inches)	(inches)	(cfs)
1.96	0.72	0.01
1.97	0.72	0.01
1.98	0.73	0.01
1.99	0.74	0.00
2.01	0.75	0.00
2.02	0.76	0.00
2.03	0.76	0.00
2.04	0.77	0.00
2.05	0.78	0.00
2.05	0.78	0.00
2.06	0.79	0.00
2.07	0.79	0.00
2.08	0.80	0.00
2.09	0.81	0.00
2.09	0.81	0.00
	(inches) 1.96 1.97 1.98 1.99 2.01 2.02 2.03 2.04 2.05 2.05 2.06 2.07 2.08 2.09	(inches) (inches) 1.96 0.72 1.97 0.72 1.98 0.73 1.99 0.74 2.01 0.75 2.02 0.76 2.03 0.76 2.04 0.77 2.05 0.78 2.06 0.79 2.07 0.79 2.08 0.80 2.09 0.81

Deer Run B_LT1

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Summary for Reach 4R: Biofilter

Inflow Area = 0.154 ac, 0.00% Impervious, Inflow Depth > 0.81" Inflow = 0.20 cfs @ 12.02 hrs, Volume= 0.010 af

Outflow = 0.19 cfs @ 12.10 hrs, Volume= 0.010 af, Atten= 5%, Lag= 4.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 1.21 fps, Min. Travel Time= 2.8 min Avg. Velocity = 0.66 fps, Avg. Travel Time= 5.0 min

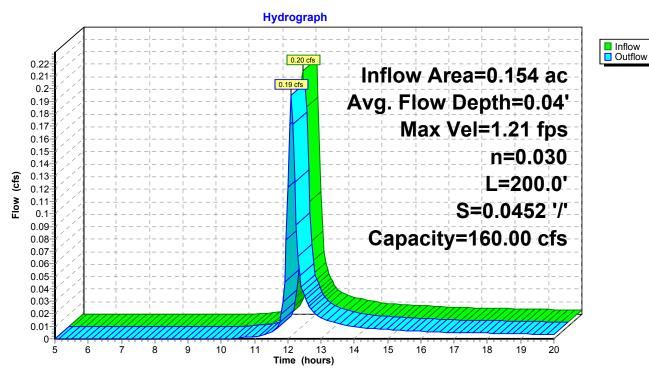
Peak Storage= 33 cf @ 12.05 hrs Average Depth at Peak Storage= 0.04'

Bank-Full Depth= 1.55' Flow Area= 15.8 sf, Capacity= 160.00 cfs

4.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 16.40' Length= 200.0' Slope= 0.0452 '/' Inlet Invert= 856.54', Outlet Invert= 847.50'



Reach 4R: Biofilter



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Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	856.54	0.00
5.50	0.00	0	856.54	0.00
6.00	0.00	0	856.54	0.00
6.50	0.00	0	856.54	0.00
7.00	0.00	0	856.54	0.00
7.50	0.00	0	856.54	0.00
8.00	0.00	0	856.54	0.00
8.50	0.00	0	856.54	0.00
9.00	0.00	0	856.54	0.00
9.50	0.00	0	856.54	0.00
10.00	0.00	0	856.54	0.00
10.50	0.00	0	856.54	0.00
11.00	0.00	1	856.54	0.00
11.50	0.01	2	856.54	0.01
12.00	0.20	30	856.58	0.12
12.50	0.03	10	856.55	0.03
13.00	0.02	5	856.55	0.02
13.50	0.01	4	856.54	0.01
14.00	0.01	3	856.54	0.01
14.50	0.01	3	856.54	0.01
15.00	0.01	3 2 2 2	856.54	0.01
15.50	0.01	2	856.54	0.01
16.00	0.01	2	856.54	0.01
16.50	0.01	2 2	856.54	0.01
17.00	0.01		856.54	0.01
17.50	0.00	1	856.54	0.00
18.00	0.00	1	856.54	0.00
18.50	0.00	1	856.54	0.00
19.00	0.00	1	856.54	0.00
19.50	0.00	1	856.54	0.00
20.00	0.00	1	856.54	0.00

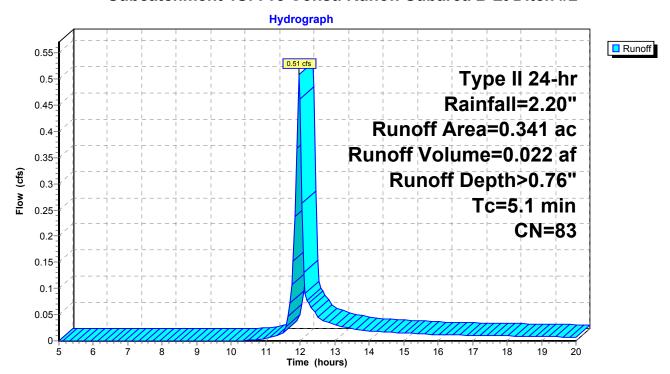
Summary for Subcatchment 1S: Pre Const. Runoff Subarea B Lt Ditch #2

Runoff = 0.51 cfs @ 11.96 hrs, Volume= 0.022 af, Depth> 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

Area	(ac)	CN	Desc	cription		
0.	.341	83	Woo	ds, Poor, I	HSG D	
0.	0.341 100.00% Pervious Area			00% Pervi	ous Area	
Tc (min)	Leng (fee		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.1						Direct Entry,

Subcatchment 1S: Pre Const. Runoff Subarea B Lt Ditch #2



Hydrograph for Subcatchment 1S: Pre Const. Runoff Subarea B Lt Ditch #2

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15 0.16	0.00	0.00
5.50 5.75	0.16	0.00	0.00 0.00
6.00	0.17	0.00	0.00
6.25	0.19	0.00	0.00
6.50	0.20	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25 8.50	0.28 0.29	0.00	0.00 0.00
8.75	0.29	0.00	0.00
9.00	0.31	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48	0.00	0.00
11.00 11.25	0.52 0.56	0.01 0.01	0.00 0.01
11.50	0.62	0.01	0.01
11.75	0.85	0.02	0.11
12.00	1.46	0.36	0.46
12.25	1.55	0.41	0.07
12.50	1.62	0.45	0.05
12.75	1.66	0.47	0.04
13.00	1.70	0.50	0.03
13.25	1.73	0.52	0.03
13.50	1.76	0.54	0.02
13.75	1.78	0.55	0.02
14.00 14.25	1.80 1.82	0.56 0.58	0.02 0.02
14.50	1.84	0.59	0.02
14.75	1.86	0.60	0.02
15.00	1.88	0.61	0.02
15.25	1.89	0.62	0.01
15.50	1.91	0.63	0.01
15.75	1.92	0.64	0.01
16.00	1.94	0.65	0.01
16.25	1.95	0.66	0.01

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.67	0.01
16.75	1.97	0.68	0.01
17.00	1.98	0.68	0.01
17.25	1.99	0.69	0.01
17.50	2.01	0.70	0.01
17.75	2.02	0.71	0.01
18.00	2.03	0.71	0.01
18.25	2.04	0.72	0.01
18.50	2.05	0.73	0.01
18.75	2.05	0.73	0.01
19.00	2.06	0.74	0.01
19.25	2.07	0.74	0.01
19.50	2.08	0.75	0.01
19.75	2.09	0.76	0.01
20.00	2.09	0.76	0.01

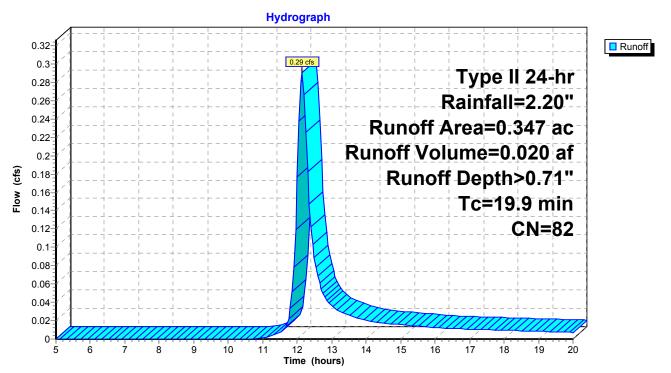
Summary for Subcatchment 3S: Post Const. Runoff Subarea B LT Ditch #2

Runoff = 0.29 cfs @ 12.14 hrs, Volume= 0.020 af, Depth> 0.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

	Area	(ac)	CN	Desc	cription			
	0.	026	77	Woo	ds, Good,	HSG D		
	0.	.149 85 Gravel roads, HSG B				HSG B		
	0.	0.172 80 >75% Grass cover, Good, HSG D					d, HSG D	
	0.	0.347 82 Weighted Average				age		
	0.	0.347 100.00% Pervious Area				ous Area		
	т.	1	41-	01	\	0 : 1	December	
	Tc	Leng	itn	Slope	Velocity	Capacity	Description	
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
	19.9						Direct Entry To	

Subcatchment 3S: Post Const. Runoff Subarea B LT Ditch #2



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Hydrograph for Subcatchment 3S: Post Const. Runoff Subarea B LT Ditch #2

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00 5.25	0.14 0.15	0.00	0.00 0.00
5.25	0.15	0.00	0.00
5.75	0.17	0.00	0.00
6.00	0.18	0.00	0.00
6.25	0.19	0.00	0.00
6.50 6.75	0.20 0.21	0.00	0.00 0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75 8.00	0.25 0.26	0.00	0.00 0.00
8.25	0.28	0.00	0.00
8.50	0.29	0.00	0.00
8.75	0.31	0.00	0.00
9.00 9.25	0.32 0.34	0.00	0.00 0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25 10.50	0.42 0.45	0.00	0.00 0.00
10.75	0.48	0.00	0.00
11.00	0.52	0.00	0.00
11.25	0.56	0.01	0.00
11.50 11.75	0.62 0.85	0.01 0.07	0.01 0.02
12.00	1.46	0.32	0.18
12.25	1.55	0.38	0.23
12.50	1.62	0.41	0.09
12.75 13.00	1.66 1.70	0.44 0.46	0.05 0.04
13.25	1.73	0.48	0.04
13.50	1.76	0.49	0.03
13.75	1.78	0.51	0.02
14.00 14.25	1.80 1.82	0.52 0.54	0.02 0.02
14.50	1.84	0.55	0.02
14.75	1.86	0.56	0.02
15.00	1.88	0.57	0.02
15.25 15.50	1.89 1.91	0.58 0.59	0.01 0.01
15.75	1.92	0.59	0.01
16.00	1.94	0.61	0.01
16.25	1.95	0.62	0.01

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.62	0.01
16.75	1.97	0.63	0.01
17.00	1.98	0.64	0.01
17.25	1.99	0.65	0.01
17.50	2.01	0.65	0.01
17.75	2.02	0.66	0.01
18.00	2.03	0.67	0.01
18.25	2.04	0.67	0.01
18.50	2.05	0.68	0.01
18.75	2.05	0.68	0.01
19.00	2.06	0.69	0.01
19.25	2.07	0.70	0.01
19.50	2.08	0.70	0.01
19.75	2.09	0.71	0.01
20.00	2.09	0.71	0.01

Deer Run B_LT2

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Summary for Reach 4R: Biofilter

Inflow Area = 0.347 ac, 0.00% Impervious, Inflow Depth > 0.71" Inflow = 0.29 cfs @ 12.14 hrs, Volume= 0.020 af

Outflow = 0.27 cfs @ 12.30 hrs, Volume= 0.020 af, Atten= 9%, Lag= 9.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 1.26 fps, Min. Travel Time= 5.7 min

Avg. Velocity = 0.62 fps, Avg. Travel Time= 11.6 min

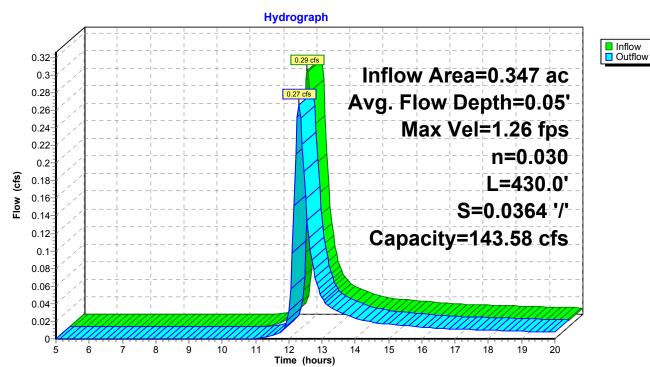
Peak Storage= 91 cf @ 12.20 hrs Average Depth at Peak Storage= 0.05'

Bank-Full Depth= 1.55' Flow Area= 15.8 sf, Capacity= 143.58 cfs

4.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 16.40' Length= 430.0' Slope= 0.0364 '/' Inlet Invert= 856.54', Outlet Invert= 840.89'



Reach 4R: Biofilter



Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	856.54	0.00
5.50	0.00	0	856.54	0.00
6.00	0.00	0	856.54	0.00
6.50	0.00	0	856.54	0.00
7.00	0.00	0	856.54	0.00
7.50	0.00	0	856.54	0.00
8.00	0.00	0	856.54	0.00
8.50	0.00	0	856.54	0.00
9.00	0.00	0	856.54	0.00
9.50	0.00	0	856.54	0.00
10.00	0.00	0	856.54	0.00
10.50	0.00	0	856.54	0.00
11.00	0.00	1	856.54	0.00
11.50	0.01	3	856.54	0.00
12.00	0.18	46	856.57	0.03
12.50	0.09	55	856.57	0.16
13.00	0.04	29	856.56	0.05
13.50	0.03	22	856.55	0.03
14.00	0.02	17	856.55	0.02
14.50	0.02	14	856.55	0.02
15.00	0.02	12	856.55	0.02
15.50	0.01	11	856.55	0.02
16.00	0.01	10	856.55	0.01
16.50	0.01	9	856.54	0.01
17.00	0.01	8	856.54	0.01
17.50	0.01	8	856.54	0.01
18.00	0.01	7	856.54	0.01
18.50	0.01	7	856.54	0.01
19.00	0.01	6	856.54	0.01
19.50	0.01	6	856.54	0.01
20.00	0.01	5	856.54	0.01

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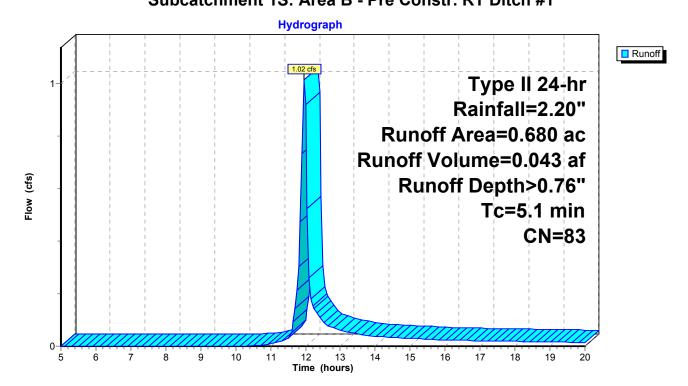
Summary for Subcatchment 1S: Area B - Pre Constr. RT Ditch #1

Runoff = 1.02 cfs @ 11.96 hrs, Volume= 0.043 af, Depth> 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

Area	(ac)	CN	Desc	cription		
0	0.658 83 Woods, Poor, HSG D					
0	.022	91	Grav	el roads, l	HSG D	
0	0.680 83 Weighted Average					
0	0.680 100.00% Pervious Area				ous Area	
_						
Tc	Leng	ith S	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
5.1						Direct Entry,

Subcatchment 1S: Area B - Pre Constr. RT Ditch #1



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Hydrograph for Subcatchment 1S: Area B - Pre Constr. RT Ditch #1

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75 6.00	0.17 0.18	0.00	0.00 0.00
6.25	0.18	0.00	0.00
6.50	0.19	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50 8.75	0.29 0.31	0.00	0.00 0.00
9.00	0.31	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75 11.00	0.48 0.52	0.00 0.01	0.00 0.01
11.00	0.52	0.01	0.01
11.50	0.62	0.01	0.02
11.75	0.85	0.08	0.21
12.00	1.46	0.36	0.92
12.25	1.55	0.41	0.14
12.50	1.62	0.45	0.09
12.75	1.66	0.47	0.07
13.00	1.70	0.50	0.06
13.25 13.50	1.73 1.76	0.52 0.54	0.05 0.05
13.75	1.78	0.55	0.03
14.00	1.80	0.56	0.04
14.25	1.82	0.58	0.04
14.50	1.84	0.59	0.03
14.75	1.86	0.60	0.03
15.00	1.88	0.61	0.03
15.25	1.89	0.62	0.03
15.50	1.91	0.63	0.03
15.75 16.00	1.92 1.94	0.64 0.65	0.03 0.02
16.25	1.95	0.66	0.02

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.67	0.02
16.75	1.97	0.68	0.02
17.00	1.98	0.68	0.02
17.25	1.99	0.69	0.02
17.50	2.01	0.70	0.02
17.75	2.02	0.71	0.02
18.00	2.03	0.71	0.02
18.25	2.04	0.72	0.02
18.50	2.05	0.73	0.02
18.75	2.05	0.73	0.02
19.00	2.06	0.74	0.02
19.25	2.07	0.74	0.02
19.50	2.08	0.75	0.02
19.75	2.09	0.76	0.01
20.00	2.09	0.76	0.01

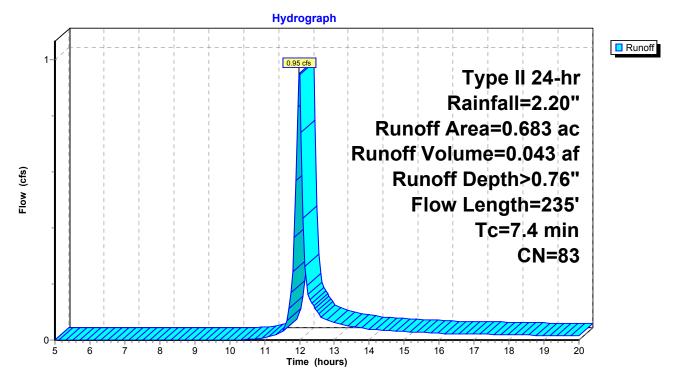
Summary for Subcatchment 3S: Area B - Post Constr. RT Ditch #1

Runoff = 0.95 cfs @ 11.99 hrs, Volume= 0.043 af, Depth> 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

_	Area	(ac)	CN	Desc	cription			
	0.160 80 >75% Grass cover, Good,					over, Good	, HSG D	
	0.083 91 Gravel roads, HSG D					HSG D		
_	0.	440	83	Woo	ds, Poor, I	HSG D		
	0.683 83 Weighted Average				hted Aver	age		
	0.683 100.00% Pervious Area							
	_							
	Tc	Leng		Slope	Velocity	Capacity	Description	
_	(min)	(fee	<u>:t)</u>	(ft/ft)	(ft/sec)	(cfs)		
	7.4	23	35		0.53		Direct Entry. Tc	

Subcatchment 3S: Area B - Post Constr. RT Ditch #1



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Hydrograph for Subcatchment 3S: Area B - Post Constr. RT Ditch #1

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25 5.50	0.15 0.16	0.00	0.00 0.00
5.75	0.10	0.00	0.00
6.00	0.17	0.00	0.00
6.25	0.19	0.00	0.00
6.50	0.20	0.00	0.00
6.75 7.00	0.21 0.22	0.00	0.00 0.00
7.25	0.22	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00 8.25	0.26 0.28	0.00	0.00 0.00
8.50	0.29	0.00	0.00
8.75	0.31	0.00	0.00
9.00	0.32	0.00	0.00
9.25	0.34	0.00	0.00
9.50 9.75	0.36 0.38	0.00	0.00 0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75 11.00	0.48 0.52	0.00 0.01	0.00 0.01
11.25	0.56	0.01	0.02
11.50	0.62	0.02	0.03
11.75	0.85	0.08	0.17
12.00	1.46	0.36 0.41	0.95 0.16
12.25 12.50	1.55 1.62	0.41	0.16
12.75	1.66	0.47	0.07
13.00	1.70	0.50	0.06
13.25	1.73	0.52	0.05
13.50 13.75	1.76 1.78	0.54 0.55	0.05 0.04
14.00	1.80	0.56	0.04
14.25	1.82	0.58	0.04
14.50	1.84	0.59	0.03
14.75 15.00	1.86 1.88	0.60 0.61	0.03 0.03
15.25	1.89	0.62	0.03
15.50	1.91	0.63	0.03
15.75	1.92	0.64	0.03
16.00 16.25	1.94 1.95	0.65 0.66	0.02 0.02
10.25	1.90	0.00	0.02

Precip.	Excess	Runoff
(inches)	(inches)	(cfs)
1.96	0.67	0.02
1.97	0.68	0.02
1.98	0.68	0.02
1.99	0.69	0.02
2.01	0.70	0.02
2.02	0.71	0.02
2.03	0.71	0.02
2.04	0.72	0.02
2.05	0.73	0.02
2.05	0.73	0.02
2.06	0.74	0.02
2.07	0.74	0.02
2.08	0.75	0.02
2.09	0.76	0.01
2.09	0.76	0.01
	(inches) 1.96 1.97 1.98 1.99 2.01 2.02 2.03 2.04 2.05 2.05 2.06 2.07 2.08 2.09	(inches) (inches) 1.96 0.67 1.97 0.68 1.98 0.69 2.01 0.70 2.02 0.71 2.03 0.71 2.04 0.72 2.05 0.73 2.05 0.73 2.06 0.74 2.07 0.74 2.08 0.75 2.09 0.76

Deer Run B_Rt1

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Summary for Reach 4R: Biofilter

Inflow Area = 0.683 ac, 0.00% Impervious, Inflow Depth > 0.76" Inflow = 0.95 cfs @ 11.99 hrs, Volume= 0.043 af

Outflow = 0.89 cfs @ 12.03 hrs, Volume= 0.043 af, Atten= 7%, Lag= 2.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 2.13 fps, Min. Travel Time= 1.4 min Avg. Velocity = 0.73 fps, Avg. Travel Time= 4.0 min

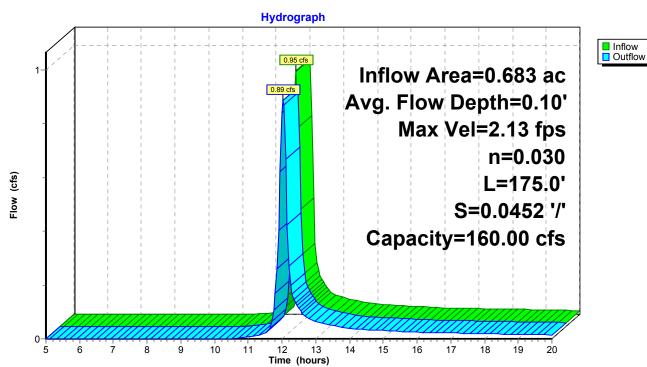
Peak Storage= 76 cf @ 12.01 hrs Average Depth at Peak Storage= 0.10'

Bank-Full Depth= 1.55' Flow Area= 15.8 sf, Capacity= 160.00 cfs

4.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 16.40' Length= 175.0' Slope= 0.0452 '/' Inlet Invert= 856.54', Outlet Invert= 848.63'



Reach 4R: Biofilter



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Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	856.54	0.00
5.50	0.00	0	856.54	0.00
6.00	0.00	0	856.54	0.00
6.50	0.00	0	856.54	0.00
7.00	0.00	0	856.54	0.00
7.50	0.00	0	856.54	0.00
8.00	0.00	0	856.54	0.00
8.50	0.00	0	856.54	0.00
9.00	0.00	0	856.54	0.00
9.50	0.00	0	856.54	0.00
10.00	0.00	0	856.54	0.00
10.50	0.00	0	856.54	0.00
11.00	0.01	2	856.54	0.01
11.50	0.03	6	856.55	0.02
12.00	0.95	76	856.64	0.84
12.50	0.10	20	856.57	0.11
13.00	0.06	14	856.56	0.07
13.50	0.05	12	856.56	0.05
14.00	0.04	11	856.56	0.04
14.50	0.03	9	856.55	0.03
15.00	0.03	8	856.55	0.03
15.50	0.03	8	856.55	0.03
16.00	0.02	7	856.55	0.02
16.50	0.02	6	856.55	0.02
17.00	0.02	6	856.55	0.02
17.50	0.02	6	856.55	0.02
18.00	0.02	5	856.55	0.02
18.50	0.02	5	856.55	0.02
19.00	0.02	5	856.55	0.02
19.50	0.02	4	856.55	0.02
20.00	0.01	4	856.55	0.01

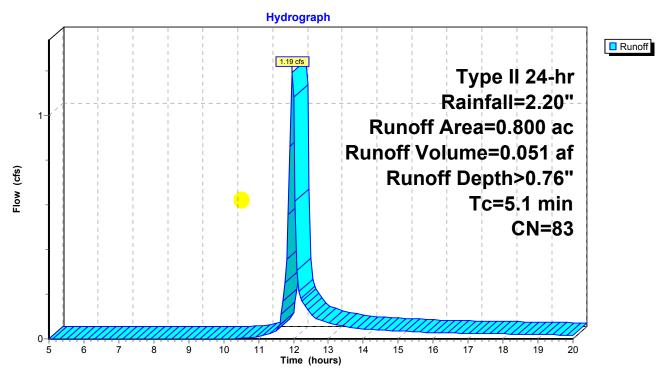
Summary for Subcatchment 1S: Pre Const. Runoff Subarea B Rt Ditch #2

Runoff = 1.19 cfs @ 11.96 hrs, Volume= 0.051 af, Depth> 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

	Area	(ac)	CN	Desc	cription		
	0.	.800	83	Woo	ds, Poor, I	HSG D	
_	0.	.800		100.	00% Pervi	ous Area	
	_						
	Tc	Leng	jth	Slope	Velocity	Capacity	Description
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
•	5 1	_					Direct Entry.

Subcatchment 1S: Pre Const. Runoff Subarea B Rt Ditch #2



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Hydrograph for Subcatchment 1S: Pre Const. Runoff Subarea B Rt Ditch #2

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75	0.17	0.00	0.00
6.00 6.25	0.18 0.19	0.00	0.00 0.00
6.50	0.19	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00	0.26	0.00	0.00
8.25	0.28	0.00	0.00
8.50 8.75	0.29 0.31	0.00	0.00 0.00
9.00	0.31	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00	0.40	0.00	0.00
10.25	0.42	0.00	0.00
10.50	0.45	0.00	0.00
10.75	0.48 0.52	0.00	0.01
11.00 11.25	0.52	0.01 0.01	0.01 0.02
11.50	0.62	0.01	0.02
11.75	0.85	0.08	0.25
12.00	1.46	0.36	1.08
12.25	1.55	0.41	0.17
12.50	1.62	0.45	0.11
12.75	1.66	0.47	0.08
13.00	1.70	0.50	0.07
13.25 13.50	1.73 1.76	0.52 0.54	0.06 0.06
13.75	1.78	0.55	0.05
14.00	1.80	0.56	0.04
14.25	1.82	0.58	0.04
14.50	1.84	0.59	0.04
14.75	1.86	0.60	0.04
15.00	1.88	0.61	0.04
15.25	1.89	0.62	0.03
15.50 15.75	1.91	0.63	0.03
16.00	1.92 1.94	0.64 0.65	0.03 0.03
16.25	1.95	0.66	0.03

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.67	0.03
16.75	1.97	0.68	0.03
17.00	1.98	0.68	0.02
17.25	1.99	0.69	0.02
17.50	2.01	0.70	0.02
17.75	2.02	0.71	0.02
18.00	2.03	0.71	0.02
18.25	2.04	0.72	0.02
18.50	2.05	0.73	0.02
18.75	2.05	0.73	0.02
19.00	2.06	0.74	0.02
19.25	2.07	0.74	0.02
19.50	2.08	0.75	0.02
19.75	2.09	0.76	0.02
20.00	2.09	0.76	0.02

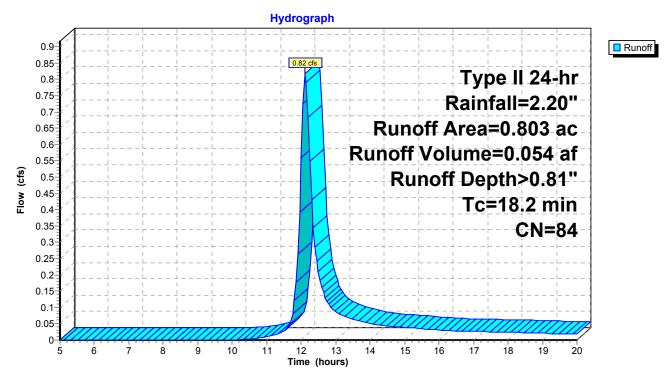
Summary for Subcatchment 3S: Post Const. Runoff Subarea B Ditch #2

Runoff = 0.82 cfs @ 12.11 hrs, Volume= 0.054 af, Depth> 0.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type II 24-hr Rainfall=2.20"

_	Area	(ac)	CN	Desc	cription		
	0.	391	83	Woo	ds, Poor, I	HSG D	
	0.	180	91	Grav	el roads, l	HSG D	
	0.	232	80	>759	% Grass co	over, Good	d, HSG D
	0.	803	84	Weig	ghted Aver	age	
	0.	803		100.	00% Pervi	ous Area	
	Tc	Leng		Slope	Velocity	Capacity	Description
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
	18 2						Direct Entry Tc

Subcatchment 3S: Post Const. Runoff Subarea B Ditch #2



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Hydrograph for Subcatchment 3S: Post Const. Runoff Subarea B Ditch #2

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
5.00	0.14	0.00	0.00
5.25	0.15	0.00	0.00
5.50	0.16	0.00	0.00
5.75 6.00	0.17 0.18	0.00	0.00 0.00
6.25	0.10	0.00	0.00
6.50	0.20	0.00	0.00
6.75	0.21	0.00	0.00
7.00	0.22	0.00	0.00
7.25	0.23	0.00	0.00
7.50	0.24	0.00	0.00
7.75	0.25	0.00	0.00
8.00 8.25	0.26 0.28	0.00	0.00 0.00
8.50	0.20	0.00	0.00
8.75	0.31	0.00	0.00
9.00	0.32	0.00	0.00
9.25	0.34	0.00	0.00
9.50	0.36	0.00	0.00
9.75	0.38	0.00	0.00
10.00 10.25	0.40	0.00	0.00
10.25	0.42 0.45	0.00	0.00 0.00
10.30	0.43	0.00	0.00
11.00	0.52	0.01	0.01
11.25	0.56	0.02	0.02
11.50	0.62	0.03	0.03
11.75	0.85	0.09	0.08
12.00	1.46	0.39	0.56
12.25	1.55	0.45	0.55
12.50 12.75	1.62 1.66	0.49 0.51	0.21 0.12
13.00	1.70	0.51	0.12
13.25	1.73	0.56	0.07
13.50	1.76	0.58	0.06
13.75	1.78	0.59	0.06
14.00	1.80	0.61	0.05
14.25	1.82	0.62	0.05
14.50	1.84	0.63	0.04
14.75 15.00	1.86 1.88	0.65 0.66	0.04 0.04
15.00	1.89	0.67	0.04
15.50	1.91	0.67	0.04
15.75	1.92	0.69	0.03
16.00	1.94	0.70	0.03
16.25	1.95	0.71	0.03

Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)
16.50	1.96	0.72	0.03
16.75	1.97	0.72	0.03
17.00	1.98	0.73	0.03
17.25	1.99	0.74	0.03
17.50	2.01	0.75	0.03
17.75	2.02	0.76	0.02
18.00	2.03	0.76	0.02
18.25	2.04	0.77	0.02
18.50	2.05	0.78	0.02
18.75	2.05	0.78	0.02
19.00	2.06	0.79	0.02
19.25	2.07	0.79	0.02
19.50	2.08	0.80	0.02
19.75	2.09	0.81	0.02
20.00	2.09	0.81	0.02

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Summary for Reach 4R: Biofilter

Inflow Area = 0.803 ac, 0.00% Impervious, Inflow Depth > 0.81" Inflow = 0.82 cfs @ 12.11 hrs, Volume= 0.054 af

Outflow = 0.75 cfs @ 12.25 hrs, Volume= 0.053 af, Atten= 8%, Lag= 7.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 1.85 fps, Min. Travel Time= 4.6 min

Avg. Velocity = 0.69 fps, Avg. Travel Time= 12.4 min

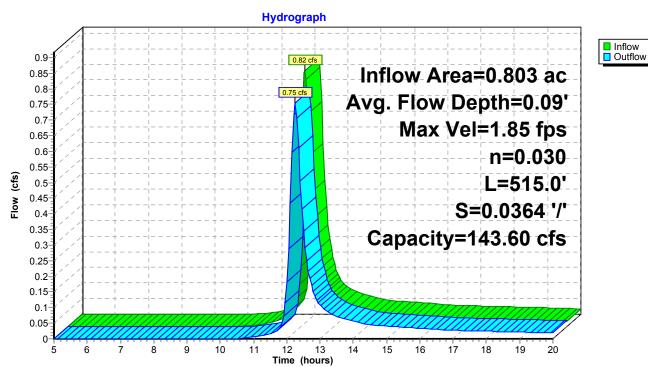
Peak Storage= 213 cf @ 12.17 hrs Average Depth at Peak Storage= 0.09'

Bank-Full Depth= 1.55' Flow Area= 15.8 sf, Capacity= 143.60 cfs

4.00' x 1.55' deep channel, n= 0.030 Side Slope Z-value= 4.0 '/' Top Width= 16.40' Length= 515.0' Slope= 0.0364 '/' Inlet Invert= 856.54', Outlet Invert= 837.79'



Reach 4R: Biofilter



Hydrograph for Reach 4R: Biofilter

Time	Inflow	Storage	Elevation	Outflow
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
5.00	0.00	0	856.54	0.00
5.50	0.00	0	856.54	0.00
6.00	0.00	0	856.54	0.00
6.50	0.00	0	856.54	0.00
7.00	0.00	Ő	856.54	0.00
7.50	0.00	Ő	856.54	0.00
8.00	0.00	Ő	856.54	0.00
8.50	0.00	0	856.54	0.00
9.00	0.00	0	856.54	0.00
9.50	0.00	0	856.54	0.00
10.00	0.00	0	856.54	0.00
10.50	0.00	1	856.54	0.00
11.00	0.01	6	856.54	0.01
11.50	0.03	16	856.55	0.02
12.00	0.56	132	856.60	0.18
12.50	0.21	109	856.59	0.34
13.00	0.09	59	856.57	0.11
13.50	0.06	46	856.56	0.07
14.00	0.05	39	856.56	0.06
14.50	0.04	36	856.56	0.05
15.00	0.04	34	856.56	0.04
15.50	0.03	32	856.56	0.04
16.00	0.03	29	856.55	0.03
16.50	0.03	26	856.55	0.03
17.00	0.03	24	856.55	0.03
17.50	0.03	23	856.55	0.03
18.00	0.02	22	856.55	0.02
18.50	0.02	20	856.55	0.02
19.00	0.02	19	856.55	0.02
19.50	0.02	18	856.55	0.02
20.00	0.02	16	856.55	0.02

Appendix D Time of Concentration Calculations

Time of Concentration - Pre Conditions Subarea A Right Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.04 hr 2.66 minutes

$$t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$$

Where,

n (Manning's roughness coefficient)	0.4
L (Sheet flow Length)	200
P ₂ (2yr, 24hr rainfall, inches)	2.63
S (slope of land surface, ft/ft)	0.06

Shallow Time of Concentration

		Depth	Manning's n	Velocity
	Minimum tillage cultivation, contour or strip-cropped, and woodlands	0.2	0.101	Use Figure 15-4 to determine Velocity
Slope	0.06	ft/ft		
Velocity from Chart	1.25	ft/s		
Length	250	ft		
t =	0.06	hr	3.33	
			minutes	

TOTAL TIME OF 0.20 hr
CONCENTRATION = 0.20 hr
12.03 minutes

Time of Concentration - Post Conditions Subarea A Right Ditch #1

Overland (Sheet Flow) Time of Concentration OR to= 0.01 hr 0.88 minutes Where, 0.4 (Manning's roughness coefficient) L 50 (Sheet flow Length) P_2 2.63 (2yr, 24hr rainfall, inches) S 0.06 (slope of land surface, ft/ft) **Shallow Time of Concentration**

		Depth	Manning's n	Velocity
Flow Type Slope Velocity from Chart Length t =		0.2 ft/ft ft/s ft hr	0.101 7.08 minutes	Use Figure 15-4 to determine Velocity
			minutes	
	Open Channel Flow			
		2	Calc Check	
Area =	0.03	ft ²	0.03	
Radius =	0.01	ft	0.01	
Slope(ft/ft) =	0.0876			
n =	0.03			
V (velocity)	0.85	ft/sec		
v (velocity)	0.85	11/360		
Drainage Area (acres) =	0.73	acres		
C (Coefficient of Runoff) =	0.3			
I =	0.11			
Q = CiA	0.024	cfs		
Ditch Width =	2	ft	Depth of water in	0.17
Dittil Width =	2	10	Ditch =	0.17
			*in inches	
Foreslope/Backslope =	3			
			Length of Ditch =	300

 t_{open} = 0.10 hr

TOTAL TIME OF CONCENTRATION =

0.23 hr OR 13.86 minutes

Time of Concentration - Pre Conditions Subarea A Right Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.04 hr 2.66 minutes

$$t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$$

Where,

n (Manning's roughness coefficient)	0.4
L (Sheet flow Length)	200
P ₂ (2yr, 24hr rainfall, inches)	2.63
S (slope of land surface, ft/ft)	0.06

Shallow Time of Concentration

		Depth	Manning's n	Velocity
	Minimum tillage cultivation, contour or strip-cropped, and woodlands	0.2	0.101	Use Figure 15-4 to determine Velocity
Slope	0.06	ft/ft		
Velocity from Chart	1.25	ft/s		
Length	250	ft		
t =	0.06	hr	3.33	
			minutes	

TOTAL TIME OF 0.20 hr
CONCENTRATION = 0.20 hr
12.03 minutes

Time of Concentration - Post Conditions Subarea A Left Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.0004 hr 0.02 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$

Where,

n 0.011

(Manning's roughness coefficient)

L 10

(Sheet flow Length)

P₂ 2.63

(2yr, 24hr rainfall, inches)

S 0.0156

	Open Channel Flow			
			Calc Check	
Area =	0.01	ft ²	0.01	
Radius =	0.01	ft	0.01	
Slope(ft/ft) =	0.0876			
n =	0.03			
V (velocity)	0.52	ft/sec		
Drainage Area (acres) =	0.16	acres		
C (Coefficient of Runoff) =	0.3			
I =	0.11			
Q = CiA	0.005	cfs		
Q = (Check) Q = VA	0.007	cfs		*Iterative process. Change depth of water until
Ditch Width =	2	ft	Depth of water in	0.08
Diteil Width =	2	10	Ditch =	0.00
			*in inches	
Foreslope/Backslope =	3			
			Length of Ditch =	250

 t_{open} = 0.13 hr

TOTAL TIME OF CONCENTRATION =

0.13 hr
OR
8.08 minutes

Time of Concentration - Pre Conditions Subarea B Left Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.03 hr 1.51 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$

Where,

S (slope of land surface, ft/ft)

Shallow Time of Concentration

0.096

		Depth	Manning's n	Velocity
• • • • • • • • • • • • • • • • • • • •	Minimum tillage cultivation, contour or strip-cropped, and woodlands	0.2	0.101	Use Figure 15-4 to determine Velocity
Slope	0.0267	ft/ft		
Velocity from Chart	0.7	ft/s		
Length	150	ft		
t =	0.06	hr	3.57	
			minutes	

 t_{open} = 0.00 hr

TOTAL TIME OF 0.08 hr
CONCENTRATION = 5.08 minutes

Time of Concentration - Post Conditions Subarea B Left Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.00 hr 0.08 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$

0.0156

Where,

n 0.03
(Manning's roughness coefficient)

L (Sheet flow Length)

P₂ 2.63
(2yr, 24hr rainfall, inches)

S (slope of land surface, ft/ft)

Open Channel Flow Calc Check ft^2 0.02 0.02 Area = 0.00 ft Radius = 0.00 Slope(ft/ft) = 0.0452 0.03 V (velocity) 0.27 ft/sec Drainage Area (acres) = 0.15 acres C (Coefficient of Runoff) = 0.3

1 = 0.11 Q = CiA 0.005 cfs Depth of water in Ditch Width = 4 ft 0.05 Ditch = *in inches Foreslope/Backslope = 4 Length of Ditch = 165

c_{open} = 0.17 hr

TOTAL TIME OF CONCENTRATION =

0.17 hr OR 10.16 minutes

Time of Concentration - Pre Conditions Subarea B Left Ditch #2

Overland (Sheet Flow) Time of Concentration

OR to = 0.03 hr 1.51 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$

Where,

(slope of land surface, ft/ft)

n 0.4

(Manning's roughness coefficient)

L
(Sheet flow Length)

P₂
(2yr, 24hr rainfall, inches)

S
(slope of lend outfoce (t/tt))

0.4

2.63

Shallow Time of Concentration

		Depth	Manning's n	Velocity
	Minimum tillage cultivation, contour or strip-cropped, and woodlands	0.2	0.101	Use Figure 15-4 to determine Velocity
Slope	0.0267	ft/ft		
Velocity from Chart	0.7	ft/s		
Length	150	ft		
t =	0.06	hr	3.57	
			minutes	

 t_{open} = 0.00 hr

TOTAL TIME OF 0.08 hr
CONCENTRATION = 5.08 minutes

Time of Concentration - Post Conditions Subarea B Left Ditch #2

Overland (Sheet Flow) Time of Concentration

OR to = 0.00 hr 0.08 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$

Where,

n 0.03
(Manning's roughness coefficient)

L (Sheet flow Length)

P₂
2.63

(2yr, 24hr rainfall, inches)

S

(slope of land surface, ft/ft)

0.0156

	Open Channel Flow			
			Calc Check	
Area =	0.03	ft ²	0.03	
Radius =	0.01	ft	0.01	
Slope(ft/ft) =	0.0364			
n =	0.03			
V (velocity)	0.36	ft/sec		
Drainage Area (acres) =	0.34	acres		
C (Coefficient of Runoff) =	0.3			
I =	0.11			
Q = CiA	0.011	cfs		
Ditch Width =	4	ft	Depth of water in	0.09
Ditch Width -		TL.	Ditch =	0.09
			*in inches	
Foreslope/Backslope =	4			
			Length of Ditch =	430

_{open} = 0.33 hr

TOTAL TIME OF CONCENTRATION =

0.33 hr
OR
19.92 minutes

Time of Concentration - Pre Conditions Subarea B Right Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.03 hr 1.51 minutes

$$t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$$

Where,

(Manning's roughness coefficient)

(Sheet flow Length)

P₂ (2yr, 24hr rainfall, inches)

S

(slope of land surface, ft/ft)

0.4

2.63

0.096

Shallow Time of Concentration

		Depth	Manning's n	Velocity
"	Minimum tillage cultivation, contour or strip-cropped, and woodlands	0.2	0.101	Use Figure 15-4 to determine Velocity
Slope	0.0267	ft/ft		
Velocity from Chart	0.7	ft/s		
Length	150	ft		
t =	0.06	hr	3.57	
			minutes	

 t_{open} = 0.00 hr

TOTAL TIME OF 0.08 hr
CONCENTRATION = 5.08 minutes

Time of Concentration - Post Conditions Subarea B Right Ditch #1

Overland (Sheet Flow) Time of Concentration

OR to = 0.02 hr 1.36 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_0^{0.5}s^{0.4}}$

Where,

n 0.4

(Manning's roughness coefficient)

L (Sheet flow Length)

P₂
(2yr, 24hr rainfall, inches)

S (slope of land surface, ft/ft)

Open Channel Flow Calc Check ft^2 0.04 0.04 Area = 0.01 ft Radius = 0.01 Slope(ft/ft) = 0.0452 0.03 n = V (velocity) 0.51 ft/sec 0.68 Drainage Area (acres) = acres C (Coefficient of Runoff) = 0.3 1 = 0.11 Q = CiA 0.022 cfs Depth of water in Ditch Width = 4 ft 0.13 Ditch = *in inches Foreslope/Backslope = 4 Length of Ditch = 185 (in feet)

open = 0.10 hr

TOTAL TIME OF CONCENTRATION =

0.12 hr OR 7.37 minutes

Time of Concentration - Pre Conditions Subarea B Right Ditch #2

Overland (Sheet Flow) Time of Concentration

OR to = 0.03 hr 1.51 minutes

 $t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$

Where,

(slope of land surface, ft/ft)

n 0.4

(Manning's roughness coefficient)

L
(Sheet flow Length)

P₂
(2yr, 24hr rainfall, inches)

S
(Alana of land surface, ft (ft))

Shallow Time of Concentration

		Depth	Manning's n	Velocity
	Minimum tillage cultivation, contour or strip-cropped, and woodlands	0.2	0.101	Use Figure 15-4 to determine Velocity
Slope	0.0267	ft/ft		
Velocity from Chart	0.7	ft/s		
Length	150	ft		
t =	0.06	hr	3.57	
			minutes	

 t_{open} = 0.00 hr

TOTAL TIME OF 0.08 hr
CONCENTRATION = 5.08 minutes

Time of Concentration - Post Conditions Subarea B Right Ditch #2

Overland (Sheet Flow) Time of Concentration

to=

0.02 hr

OR 1.36 minutes

$$t_o = \frac{0.007(nL)^{0.8}}{P_2^{0.5}s^{0.4}}$$

Where,

n 0.4
(Manning's roughness coefficient)
L (Sheet flow Length)
P₂ 2.63
(2yr, 24hr rainfall, inches)
S (slope of land surface, ft/ft)

	Open Channel Flow			
			Calc Check	
Area =	0.05	ft ²	0.05	
Radius =	0.01	ft	0.01	
Slope(ft/ft) =	0.0364			
n =	0.03			
V (velocity)	0.51	ft/sec		
Drainage Area (acres) =	0.79	acres		
C (Coefficient of Runoff) =	0.3			
I =	0.11			
Q = CiA	0.026	cfs		
Ditch Width =	4	ft	Depth of water in Ditch =	0.15
			*in inches	
Foreslope/Backslope =	4			
			Length of Ditch =	510

open =

0.28 hr

TOTAL TIME OF CONCENTRATION =

0.30 hr OR 18.16 minutes

Appendix E Exhibits

